

STORMWATER DRAIN

[A] INPUT DATA:

(a) Site data:

- (i) Max. rainfall intensity, $I = 152.4 \text{ mm/hr}$
- (ii) Area to be drained, $A = 10000 \text{ sq.m}$
- (iii) Site surface type: 5. Clay (0.60) \Rightarrow Run-off coeff., $C = 0.6$

(b) Drain details:

- (i) Shape: Trapezoidal
- (ii) Surface type: 5. Monolithic concrete (0.0145) \Rightarrow Manning's coeff., $N = 0.0145$
- (iii) Slope: 1 in 250
- (iv) Free board, $FB = 150 \text{ mm}$
- (v) Base width, $b_1 = 300 \text{ mm}$
- (vi) Trench depth, $D = 600 \text{ mm}$
- (vii) Trench side slope, $V:H = 2:1$
- (viii) Depth of water, $d = D - FB = 600 - 150 = 450 \text{ mm}$
- (ix) Top width, $b_2 = 300 + 2 \times 1 \times 450 / 2 = 750 \text{ mm}$

[B] CALCULATIONS:

Calculated discharge, $Q_{act} = C.I.A = 0.6 \times 152.4 \times 10000 / (1000 \times 60 \times 60) = 0.254 \text{ cu.m/s}$

Area of c/s of flow, $a = (d/2) \cdot (b_1 + b_2) = 0.236 \text{ sq.m}$

Wetted perimeter, $P = 1.31 \text{ m}$

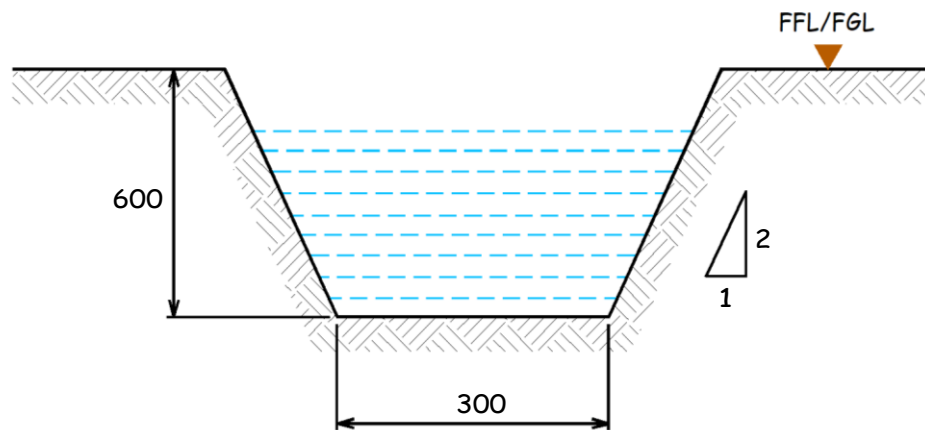
Hydraulic radius, $R = a / P = 0.18 \text{ m}$

Velocity (Manning's formula), $V = (1/N) \times R^{(2/3)} \times s^{(1/2)}$


$$\Rightarrow V = (1/0.0145) \times 0.18^{(2/3)} \times (1/250)^{(1/2)} = 1.39 \text{ m/s}$$

Discharge, $Q_{cap} = a \times V$

$$\Rightarrow Q_{cap} = 0.236 \times 1.39 = 0.328 \text{ cu.m/s} > Q_{act} (= 0.254 \text{ cu.m/s}), \text{ Hence OK}$$



DRAIN C/S (DIM IN MM)
(Scale: NTS)

	Client : EPICENTER CONSULTING ENGINEERS					Element: 11A
	Project:	1001	Doc. No.:	1001-CAL-ST-11		Location/ 1A Designation: 11A
Rev.	Ppd. by	Date	Chd. by	Date		
Project:	Infra_SWD	2				Sht. 1 of 1
Structure:	Infra_SWD	1				
Type:	SWD	0	-	5/01/2026	-	